# DYNAMIC DEFORMATION OF A CURVED 

## PLATE WITH A RIGID INSERT

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#### Abstract

A general solution is obtained for dynamic bending of ideal rigid-plastic plates with a clamped or simply supported curved contour containing an absolutely rigid insert of an arbitrary shape. The plate is affected by a short-time high-intensity explosive dynamic load uniformly distributed over the surface. It is shown that there are several mechanisms of plate deformation. Equations for dynamic deformation are derived for each mechanism, and conditions of occurrence are analyzed. Examples of numerical solutions are given.


Key words: rigid-plastic plate, arbitrary contour, rigid insert, dynamic load, ultimate load, final flexure.

Introduction. The issues of calculating structures under the action of intense short-time loads are very important in modern mechanics of deformable solids. To solve such problems, the model of a rigid-plastic body is widely used [1]. The model is based on the assumption that the body starts deforming when the stress reaches the ultimate value and plastic deformation becomes possible. Elastic deformations are neglected. For thin-sheet structural elements, this simplification allowed solving numerous issues of practical importance. The model of a rigid-plastic body was used in [2-9] to study the behavior of homogeneous plates with a complicated external contour under the action of arbitrary dynamic loads of high intensity.

Structurally inhomogeneous plates are constitutive elements of many structures used in various areas of engineering. Flat shields are often equipped by reinforced closed technological hatches. Therefore, damage of plates with rigid inserts has to be examined. Up to now, this problem has been considered only for a circular plate with a rigid circle at the center under conditions of axisymmetric loading and attachment [10]. The method proposed in the present work allows, on the basis of the theory of an ideal rigid-plastic body, calculating plates with an arbitrary curved contour, which are attached in an arbitrary manner, have an absolutely rigid insert of an arbitrary shape, and are subjected to intense short-time dynamic loads. The method can be used for a wide class of approximate engineering calculations.

1. We consider a plate made of an ideal rigid-plastic material with an arbitrary smooth convex contour $l$, which is clamped or simply supported (Fig. 1). In the central part, the plate has an absolutely rigid insert $Z_{a}$ with an arbitrary contour $l_{2}$. The plate is subjected to a high-intensity dynamic load $P(t)$ uniformly distributed over the surface. We consider explosive loads characterized by instantaneous reaching the maximum value $P_{\max }=P\left(t_{0}\right)$ at the initial time $t_{0}$ with their subsequent rapid decrease. As the insert $Z_{a}$ remains rigid during its deformation, we assume that the ultimate flexural moment in the insert is greater than $M_{0}$ (the ultimate flexural moment in the remaining part of the plate) and $\rho_{a} / \rho \geqslant 1$, where $\rho$ and $\rho_{a}$ are the surface densities of the plate and insert materials, respectively.

The dynamics of the plate made of a rigid-plastic material can follow one of the three schemes of deformation, depending on the value of $P_{\text {max }}$. Under loads lower than the ultimate values (low loads), the plate remains at rest. Under loads only slightly higher than the ultimate values (medium loads), the plate is deformed into a certain line

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Fig. 1


Fig. 2


Fig. 3
surface, whereas the absolutely rigid insert and the points of its contour move translationally with an identical velocity $\dot{w}_{c}(t)$. As in the case of a plate without the insert [4, 6-9], a plastic pivot line $l_{1}$ can be formed in the plate; this line may consist of several segments (see Fig. 1). The position of the line $l_{1}$ is determined by the shape of the support contour of the plate from the condition of identical distances from the contour $l$ to the line $l_{1}$ in the direction normal to the external contour $[7,9]$. This scheme of deformation is called scheme No. 1. As in the case of flexure of beams [1], circular and annular plates [12-14], rectangular and polygonal plates [1-3, 11], and plates with a sophisticated contour [4-9], the plate dynamics in the case of rather high values of $P_{\max }$ can be accompanied by the emergence of a zone of intense plastic deformation $Z_{p}$ moving translationally. There are also possible situations where some part of the pivot $l_{1}$ is retained or the zone $Z_{p}$ does not cover the entire insert $Z_{a}$ (high loads; scheme in Fig. 2 is called scheme No. 2) or where the pivot $l_{1}$ is absent and the insert $Z_{a}$ is inside the zone $Z_{p}$ (superhigh loads; scheme in Fig. 3 is called scheme No. 3).

Let the equation for the plate contour $l$ be set in a parametric form: $x=x_{1}(\varphi)$ and $y=y_{1}(\varphi)(0 \leqslant \varphi \leqslant 2 \pi)$. In all schemes of deformation, the normal to the curve $l$ directed inward the region occupied by the plate hits the pivot $l_{1}$, the contour $l_{2}$, or the curve $l_{3}$, which is the contour of the zone $Z_{p}$ (see Figs. 1-3). We use the notation $Z_{i j}$ to denote the zone of the plate, the normal from each point of this zone to the contour $l$ being incident onto the curve $l_{i}(i=1,2,3 ; j=1, \ldots)$. The number of the curves $l_{i}$ depends on the shapes of the plate and the insert. We denote part of the external contour $l_{i j}$, which is the support contour of the zone $Z_{i j}$, by $l_{i j}$. Part of the contour $l_{i j}$ is determined in the interval $\psi_{i j} \leqslant \varphi \leqslant \xi_{i j}(i=1,2,3 ; j=1, \ldots)$. We use the notation $D_{n j}$ to denote the distance normal to the contour $l$, calculated from the contour $l_{n j}$ to the curve $l_{n}$ in the zone $Z_{n j}(n=1,2)$. The values of $D_{n j}$ depend on the shapes of the plate and insert contours, hence, on the parameter $\varphi$ only. It can be shown
$[4,6]$ that the normal to the contour $l$ is also the normal to the curve $l_{3}$ and that the distance $D$ between $l_{3 j}$ and $l_{3}$ is independent of the parameter $\varphi$ and the subscript $j$. The equation $l_{3}\left[x=x_{3}(\varphi)\right.$ and $y=y_{3}(\varphi)$, where $\left.\psi_{3 j} \leqslant \varphi \leqslant \xi_{3 j}\right]$ for the contour of the zone $Z_{p}$ has the form $[4,6]$

$$
\begin{equation*}
x_{3}=x_{1}-D y_{1}^{\prime} / L, \quad y_{3}=y_{1}+D x_{1}^{\prime} / L \tag{1}
\end{equation*}
$$

Here $L(\varphi)=\sqrt{x_{1}^{\prime 2}(\varphi)+y_{1}^{\prime 2}(\varphi)}$, where $(\cdot)^{\prime}=\partial(\cdot) / \partial \varphi$.
We derive the equations of plate motion from the principle of virtual powers with the use of the d'Alembert principle [11]:

$$
\begin{gather*}
K=A-N  \tag{2}\\
K=\iint_{S \backslash Z_{a}} \rho \frac{\partial^{2} u}{\partial t^{2}} \frac{\partial u^{*}}{\partial t} d s+\iint_{Z_{a}} \rho_{a} \frac{\partial^{2} u}{\partial t^{2}} \frac{\partial u^{*}}{\partial t} d s \\
A=\iint_{S} P(t) \frac{\partial u^{*}}{\partial t} d s, \quad N=\sum_{m} \int_{l_{m}} M_{m}\left[\frac{\partial \theta_{m}^{*}}{\partial t}\right] d l . \tag{3}
\end{gather*}
$$

Here $K, A$, and $N$ are the powers of inertial, external, and internal forces of the plate, respectively, $S$ is the plate area, $u$ is the flexure, $t$ is the current time, $l_{m}$ are the lines of discontinuity of angular velocities, $M_{m}$ is the flexural moment on $l_{m}$, and $\left[\partial \theta_{m}^{*} / \partial t\right]$ is the discontinuity of angular velocity on $l_{m}$. In the expression for $N$, summation is performed over all lines of discontinuity of angular velocity, including the plate boundary. Admissible velocities are indicated by the asterisk.

As the zones $Z_{a}$ and $Z_{p}$ move translationally, the flexural velocity in the zone $Z_{p}$ is $\dot{w}_{c}(t)$ because of continuity of velocities at the boundaries of these zones. We denote the velocities of the angle of deflection of the zone $Z_{i j}$ on the support contour $l_{i j}$ by $\dot{\alpha}_{i j}(i=1,2,3 ; j=1, \ldots)$. The condition of continuity of velocities at the boundaries of the zones $Z_{3 j}$ and $Z_{p}$ yields $\dot{w}_{c}=\dot{\alpha}_{3 j} D$; hence, $\dot{\alpha}_{3 j}$ is independent of the parameter $\varphi$ and the subscript $j$. We denote $\dot{\alpha}_{3 j}$ by $\dot{\alpha}$. Then, the flexural velocities in different zones of the plate can be presented as

$$
\begin{gather*}
(x, y) \in Z_{a}: \quad \dot{u}(x, y, t)=\dot{w}_{c}(t), \quad(x, y) \in Z_{p}: \quad \dot{u}(x, y, t)=\dot{w}_{c}(t), \\
(x, y) \in Z_{3 j}: \quad \dot{u}(x, y, t)=\dot{\alpha}(t) d_{3 j}(x, y) \quad(j=1, \ldots)  \tag{4}\\
(x, y) \in Z_{n m}: \quad \dot{u}(x, y, t)=\dot{\alpha}_{n m}(t, \varphi) d_{n m}(x, y) \quad(n=1,2 ; m=1, \ldots),
\end{gather*}
$$

where $d_{i j}(x, y)$ is the distance from the point $(x, y)$ to the support contour of the zone $Z_{i j}(i=1,2,3 ; j=1, \ldots)$, and the dot over the symbols indicates their derivatives with respect to time.

As in [9], we assume that $\dot{\alpha}_{1 j}$ is independent of the parameter $\varphi$ but may depend on the subscript $j$. From the condition of continuity of velocities at the boundaries of the contacting zones $\left[Z_{1 i}\right.$ and $Z_{2 j} ; Z_{3 m}$ and $Z_{k n}$ $(i, j, m, n,=1, \ldots ; k=1,2)]$, we obtain the relations

$$
\begin{gather*}
\dot{\alpha}_{1 i}(t) D_{1 i}\left(\varphi_{*}\right)=\left[\dot{\alpha}_{2 j}\left(t, \varphi_{*}\right) D_{2 j}\left(\varphi_{*}\right)\right]^{\mu_{i j}}(\dot{\alpha} D)^{\lambda_{1 i}}  \tag{5}\\
\dot{\alpha}_{2 n}\left(t, \varphi_{* *}\right) D_{2 n}\left(\varphi_{* *}\right)=\left[\dot{\alpha}_{1 m}(t) D_{1 m}\left(\varphi_{* *}\right)\right]^{\mu_{m n}}(\dot{\alpha} D)^{\lambda_{2 n}} \tag{6}
\end{gather*}
$$

where $\varphi_{*}$ and $\varphi_{* *}$ are the parameters of the zone boundaries considered; $\mu_{i j}=1$ if the zones $Z_{1 i}$ and $Z_{2 j}$ are in contact and $\mu_{i j}=0$ if the zones $Z_{1 i}$ and $Z_{2 j}$ do not contact each other; $\lambda_{k j}=1$ if the zones $Z_{3 i}$ and $Z_{k j}$ are in contact and $\lambda_{k j}=0$ if the zones $Z_{3 i}$ and $Z_{k j}$ do not contact each other. It follows from equalities (5) and (6) that

$$
\begin{equation*}
\dot{\alpha}_{i j}(t, \varphi)=F_{i j}(t, \varphi) \dot{\alpha}(t) \quad(i=1,2 ; \quad j=1, \ldots) \tag{7}
\end{equation*}
$$

The power of the internal forces in Eq. (3) is calculated by the formula (see [15])

$$
N=M_{0}(2-\eta) \oint_{l} \frac{\partial \dot{u}^{*}}{\partial n} d l
$$

where $\eta=0$ if the external contour is clamped and $\eta=1$ if the external contour is simply supported; $\partial \dot{u} / \partial n$ is the derivative of the flexural velocity along the normal to the contour $l$ or the velocity of the angle of deflection of the plate surface from the horizontal line on the contour $l ; d l$ is an element of the contour $l$.

With allowance for the notation used and for the equality $F_{3 j}=1$, Eqs. (3) become

$$
\begin{gathered}
K=\dot{\alpha}^{*} \ddot{\alpha} \rho \sum_{i, j} \iint_{Z_{i j}} F_{i j}^{2} d_{i j}^{2} d s+\dot{w}_{c}^{*} \ddot{w}_{c}\left[\rho \iint_{Z_{p}} d s+\rho_{a} \iint_{Z_{a}} d s\right], \\
A=P(t)\left[\dot{\alpha}^{*} \sum_{i, j} \iint_{Z_{i j}} F_{i j} d_{i j} d s+\dot{w}_{c}^{*} \iint_{Z_{a} \cup Z_{p}} d s\right], \quad N=M_{0}(2-\eta) \dot{\alpha}^{*} \sum_{i, j} \int_{l_{i j}} F_{i j} d l .
\end{gathered}
$$

Substituting these equalities into (2) and taking into account that $\dot{w}_{c}^{*}(t)$ and $\dot{\alpha}^{*}(t)$ are independent of each other, we obtain the equations of motion for the deformation scheme No. 2:

$$
\begin{gather*}
\rho \ddot{\alpha} \sum_{i, j} \iint_{Z_{i j}} F_{i j}^{2} d_{i j}^{2} d s=P(t) \sum_{i, j} \iint_{Z_{i j}} F_{i j} d_{i j} d s-M_{0}(2-\eta) \sum_{i, j} \int_{l_{i j}} F_{i j} d l ;  \tag{8}\\
\ddot{w}_{c}\left[\rho \iint_{Z_{p}} d s+\rho_{a} \iint_{Z_{a}} d s\right]=P(t) \iint_{Z_{a} \cup Z_{p}} d s . \tag{9}
\end{gather*}
$$

The condition of continuity of velocities at the boundaries of the zones $S_{p}$ and $Z_{3 j}$ yields the equality

$$
\begin{equation*}
\dot{\alpha} D=\dot{w}_{c} . \tag{10}
\end{equation*}
$$

At the boundaries of the contacting zones $Z_{i j}$ and $Z_{m n}(i, m=1,2)$, the following condition is satisfied:

$$
\begin{equation*}
D_{i j}\left(\beta_{i j m n}\right)=D_{m n}\left(\beta_{i j m n}\right) . \tag{11}
\end{equation*}
$$

Here the parameter $\beta_{i j m n}$ determines the boundary of the zones $Z_{i j}$ and $Z_{m n}\left(\beta_{i j m n}=\psi_{i j}\right.$ or $\left.\beta_{i j m n}=\xi_{i j}\right)$.
The boundaries of the zones $Z_{i j}(i=1,2)$ and $Z_{3 n}$ obey the equality

$$
\begin{equation*}
D=D_{i j}\left(\varphi_{i j}\right), \tag{12}
\end{equation*}
$$

where $\varphi_{i j}=\psi_{i j}$ or $\varphi_{i j}=\xi_{i j}$.
At the initial time, the plate is at rest, i.e.,

$$
\begin{equation*}
\alpha\left(t_{0}\right)=\dot{\alpha}\left(t_{0}\right)=w_{c}\left(t_{0}\right)=\dot{w}_{c}\left(t_{0}\right)=0 \tag{13}
\end{equation*}
$$

The initial values of $D\left(t_{0}\right)$ and $\beta_{i j m n}\left(t_{0}\right)$ are determined depending on the value of $P_{\max }$, which will be demonstrated below for particular problems.

System (7)-(12) describes the plate motion for the case of deformation by scheme No. 2. In the case of deformation by scheme No. 3, there are no zones $Z_{1 i}$ and $Z_{2 j}$, the zones $Z_{3 n}$ merge into one zone, and the motion is described by Eqs. (8)-(10) with $i=3$.

In the case of deformation by scheme No. 1 , there are no zones $Z_{3 n}$; the equalities

$$
\begin{equation*}
\dot{\alpha}_{i j}(t, \varphi)=G_{i j} \dot{w}_{c}(t), \quad G_{1 j}=\frac{1}{D_{1 j}\left(\varphi_{*}\right)}, \quad G_{2 j}(\varphi)=\frac{1}{D_{2 j}(\varphi)} \quad(i=1,2 ; j=1, \ldots) \tag{14}
\end{equation*}
$$

are valid instead of Eqs. (7) and (1) and expressions (3) acquire the form

$$
\begin{gathered}
K=\dot{w}_{c}^{*} \ddot{w}_{c}\left(\rho \sum_{i=1,2} \sum_{j} \iint_{Z_{i j}} G_{i j}^{2} d_{i j}^{2} d s+\rho_{a} \iint_{Z_{a}} d s\right), \\
A=\dot{w}_{c}^{*} P(t)\left(\sum_{i=1,2} \sum_{j} \int_{Z_{i j}} \int_{i j} G_{i j} d_{i j} d s+\iint_{Z_{a}} d s\right), \quad N=\dot{w}_{c}^{*} M_{0}(2-\eta) \sum_{i=1,2} \sum_{j} \int_{l_{i j}} G_{i j} d l .
\end{gathered}
$$

Substituting these equalities into Eq. (2), we obtain

$$
\begin{gather*}
\ddot{w}_{c}\left(\rho \sum_{i=1,2} \sum_{j} \iint_{Z_{i j}} G_{i j}^{2} d_{i j}^{2} d s+\rho_{a} \iint_{Z_{a}} d s\right) \\
=P(t)\left(\sum_{i=1,2} \sum_{j} \iint_{Z_{i j}} G_{i j} d_{i j} d s+\iint_{Z_{a}} d s\right)-M_{0}(2-\eta) \sum_{i=1,2} \sum_{j} \int_{l_{i j}} G_{i j} d l . \tag{15}
\end{gather*}
$$



Fig. 4
System (14), (15) describes the plate motion for the case of deformation by scheme No. 1. Deflections in different zones of the plate are determined by Eqs. (4).

To calculate double integrals over the zones $Z_{i j}(i=1,2,3 ; j=1, \ldots)$ in the equations of motion, it is convenient to pass to a curvilinear coordinate system ( $\nu_{1}, \nu_{2}$ ) related to the Cartesian coordinate system as

$$
\begin{equation*}
x=x_{1}\left(\nu_{2}\right)-\nu_{1} y_{1}^{\prime}\left(\nu_{2}\right) / L\left(\nu_{2}\right), \quad y=y_{1}\left(\nu_{2}\right)+\nu_{1} x_{1}^{\prime}\left(\nu_{2}\right) / L\left(\nu_{2}\right) \tag{16}
\end{equation*}
$$

The coordinate lines $\nu_{1}=$ const are located at a distance $\nu_{1}$ from the contour $l$. The straight lines $\nu_{2}=$ const are normals to the external contour of the plate.

We determine the ultimate load $P_{0}$ from Eq. (15) at the time of motion beginning $t_{0}$ and from the condition $\ddot{w}_{c}\left(t_{0}\right)=0$. Then, we have

$$
\begin{equation*}
P_{0}=\frac{M_{0}(2-\eta) \sum_{i=1,2} \sum_{j} \int_{l_{i j}} G_{i j} d l}{\sum_{i=1,2} \sum_{j} \iint_{Z_{i j}} G_{i j} d_{i j} d s+\iint_{Z_{a}} d s} \tag{17}
\end{equation*}
$$

2. We consider the dynamic motion of curved plates with an arbitrary rigid insert by an example of a circular plate with a regular $n$-angle rigid insert located at the center. Let $R$ and $R_{1}$ be the radii of the circular plate and the circumference inscribed into the polygonal contour of the rigid insert, respectively; $R_{1}<R \cos (\pi / n)$ (Fig. 4). Under medium loads, the plate is deformed into a line surface with formation (owing to its symmetry) of $n$ identical zones $Z_{2}$, i.e., we have scheme No. 1 (see Fig. 4); the velocity of the angle of rotation on the support contour is $\dot{\alpha}_{2}(t, \varphi)$. Under high loads, $n$ identical zones $Z_{p}$ are formed in the central part of the plate, in the vicinity of the rigid insert, i.e., we have scheme No. 2 (Fig. 5). Equation (1) for $l_{3}$ of the contour $Z_{p}$ in the polar coordinate system has the form

$$
x_{3}=(R-D) \cos \varphi, \quad y_{3}=(R-D) \sin \varphi \quad(-\xi \leqslant \varphi \leqslant \xi, \quad 0<\xi<\pi / n)
$$

There are still $n$ identical zones $Z_{2}: \xi \leqslant \varphi \leqslant(2 \pi / n)-\xi$. Under superhigh loads, there are no zones $Z_{2}$, whereas the insert $Z_{a}$ is located inside the zone $Z_{p}$ and $D \leqslant R-R_{1} / \cos (\pi / n)$, i.e., we have scheme No. 3 (Fig. 6).

In this case, the equations of plate motion (7)-(9), (11), and (12) for the scheme of deformation No. 2 are

$$
\begin{gather*}
\rho \ddot{\alpha} \Sigma_{1}=P(t) \Sigma_{2}-M_{0}(2-\eta) \Sigma_{3}  \tag{18}\\
\ddot{w}_{c}\left(\rho \Sigma_{4}+\rho_{a} \Sigma_{5}\right)=P(t) \Sigma_{6}  \tag{19}\\
D=R-R_{1} / \cos \xi  \tag{20}\\
\dot{\alpha}_{2}(t, \varphi)=F_{2}(t, \varphi) \dot{\alpha}(t) ; \quad F_{2}(t, \varphi)=\left[R-R_{1} / \cos \xi(t)\right] /\left(R-R_{1} / \cos \varphi\right) \tag{21}
\end{gather*}
$$



Fig. 5


Fig. 6

Here

$$
\begin{gathered}
\Sigma_{1}(\xi)=\frac{n\left(R \cos \xi-R_{1}\right)^{2}}{6 \cos ^{2} \xi}\left\{\xi \frac{\left(R \cos \xi-R_{1}\right)\left(R \cos \xi+3 R_{1}\right)}{\cos ^{2} \xi}+R^{2}\left(\frac{\pi}{n}-\xi\right)\right. \\
\left.-3 R_{1}^{2}\left(\tan \frac{\pi}{n}-\tan \xi\right)+2 R R_{1}\left[\ln \frac{\cos (\pi / n)}{1-\sin (\pi / n)}-\ln \frac{\cos \xi}{1-\sin \xi}\right]\right\} ; \\
\Sigma_{2}(\xi)=\frac{n\left(R \cos \xi-R_{1}\right)}{3 \cos \xi}\left\{\xi \frac{\left(R \cos \xi-R_{1}\right)\left(R \cos \xi+2 R_{1}\right)}{\cos ^{2} \xi}+R^{2}\left(\frac{\pi}{n}-\xi\right)\right. \\
\left.+R R_{1}\left[\ln \frac{\cos (\pi / n)}{1-\sin (\pi / n)}-\ln \frac{\cos \xi}{1-\sin \xi}\right]-2 R_{1}^{2}\left(\tan \frac{\pi}{n}-\tan \xi\right)\right\} ; \\
\Sigma_{3}(\xi)=2 n R\left[\xi+\frac{R \cos \xi-R_{1}}{R \cos \xi}\left(\frac{\pi}{n}-\xi+\frac{R_{1}}{A_{*}} \Sigma_{*}\right)\right] ; \quad \Sigma_{4}(\xi)=n R_{1}^{2}\left(\frac{\xi}{\cos ^{2} \xi}-\tan \xi\right) ; \\
\Sigma_{5}=n R_{1}^{2} \tan \frac{\pi}{n} ; \quad \Sigma_{6}(\xi)=\Sigma_{4}(\xi)+\Sigma_{5}=n R_{1}^{2}\left(\frac{\xi}{\cos ^{2} \xi}-\tan \xi+\tan \frac{\pi}{n}\right) ; \\
\Sigma_{*}=\ln \frac{\left[B_{*} \tan (\pi / 2 n)+A_{*}\right]\left[-B_{*} \tan (\xi / 2)+A_{*}\right]}{\left[-B_{*} \tan (\pi / 2 n)+A_{*}\right]\left[B_{*} \tan (\xi / 2)+A_{*}\right]} ; \quad B_{*}=R+R_{1} ; \quad A_{*}=\sqrt{R^{2}-R_{1}^{2}} .
\end{gathered}
$$

Then, the plate motion by scheme No. 2 is described by Eqs. (10) and (18)-(21).
The equations of the plate motion (14) and (15) by scheme No. 1 have the form

$$
\begin{gather*}
\dot{\alpha}_{2}(t, \varphi)=G_{2}(\varphi) \dot{w}_{c}(t) ; \quad G_{2}(\varphi)=\cos \varphi /\left(R \cos \varphi-R_{1}\right) ;  \tag{22}\\
\ddot{w}_{c}\left(\rho \Sigma_{7}+\rho_{a} \Sigma_{5}\right)=P(t) \Sigma_{8}-M_{0}(2-\eta) \Sigma_{9}, \tag{23}
\end{gather*}
$$

where

$$
\Sigma_{7}=\Sigma_{1}(0) /\left(R-R_{1}\right)^{2} ; \quad \Sigma_{8}=\Sigma_{2}(0) /\left(R-R_{1}\right)+\Sigma_{5} ; \quad \Sigma_{9}=\Sigma_{3}(0) /\left(R-R_{1}\right)
$$

The equations of the plate motion (8) and (9) by scheme No. 3 have the form

$$
\begin{gather*}
\rho \ddot{\alpha} \Sigma_{10}=P(t) \Sigma_{11}-M_{0}(2-\eta) \Sigma_{12} ;  \tag{24}\\
\ddot{w}_{c}\left(\rho \Sigma_{13}+\rho_{a} \Sigma_{5}\right)=P(t) \Sigma_{14} . \tag{25}
\end{gather*}
$$



Fig. 7

Here

$$
\begin{gathered}
\Sigma_{10}(D)=D^{3}(4 R-3 D) / 6 ; \quad \Sigma_{11}(D)=D^{2}(3 R-2 D) / 3 ; \quad \Sigma_{12}=\Sigma_{3}(\pi / n) / \pi=2 R ; \\
\Sigma_{13}(D)=\pi(R-D)^{2}-n R_{1}^{2} \tan (\pi / n) ; \quad \Sigma_{14}(D)=\Sigma_{13}(D)+\Sigma_{5}=\pi(R-D)^{2} .
\end{gathered}
$$

Then, the plate motion by scheme No. 3 is described by Eqs. (10), (24), and (25).
The ultimate load $P_{0}$ (17) is determined as

$$
\begin{equation*}
P_{0}=\frac{M_{0}(2-\eta) \Sigma_{9}}{\Sigma_{8}}=\frac{M_{0}(2-\eta) \Sigma_{3}(0)}{\Sigma_{2}(0)+\left(R-R_{1}\right) \Sigma_{5}} . \tag{26}
\end{equation*}
$$

For $R_{1}=0$, the plate considered becomes circular and does not contain any insert. For this case, Eq. (26) yields $P_{0}=6 M_{0}(2-\eta) / R^{2}$. If the plate is simply supported, this value equals the ultimate load $\bar{P}_{0}$ obtained in [16] on the basis of the exact solution. In the case of a clamped contour, the ultimate load predicted by Eq. (26) is $2 \bar{P}_{0}$. In [13], it was obtained as a result of an approximate solution on the basis of Tresca's condition of plasticity and equals $1.875 \bar{P}_{0}$. As $n \rightarrow \infty$, we obtain a circular rigid insert. For such a plate, the ultimate load is determined as

$$
\begin{equation*}
P_{0}=6 M_{0}(2-\eta) R /\left(R^{3}-R_{1}^{3}\right) \tag{27}
\end{equation*}
$$

and coincides, in the case of a simply supported plate, with the ultimate load obtained on the basis of the exact solution [10]. Figure 7 shows the load $p_{0}=P_{0} R^{2} /\left[(2-\eta) M_{0}\right]$ as a function of the ratio $R_{1} / R$ for different values of $n: n=3$ (curve 1 ), $n=4$ (curve 2 ), $n=5$ (curve 3 ), and $n=\infty$ (curve 4).

We optimize the shape of a regular $n$-angle rigid insert to find the extreme value of the ultimate load of the plate considered under the condition of a constant area of the rigid insert $S_{a}$, thickness, method of attachment, and radius of the circular plate. As the area of the rigid insert is $S_{a}=n R_{1}^{2} \tan (\pi / n)$, Eq. (26) acquires the form

$$
\begin{gathered}
P_{0}=6 M_{0}(2-\eta) A_{1} / R^{2}, \\
A_{1}=\frac{\frac{\pi}{n}+\frac{\delta}{\pi / n+\delta^{2}} \operatorname{lan}(\pi / n)+\delta \ln \{\cos (\pi / n) /[1-\sin (\pi / n)]\}}{-(1+\delta) \tan (\pi / 2 n)+\sqrt{1-\delta^{2}}} \\
\end{gathered} . \quad \delta=\frac{1}{R} \sqrt{\frac{S_{a}}{n \tan (\pi / n)}} .
$$

As $A_{1}$ decreases with increasing $n$ and the inequality $R_{1}=\sqrt{S_{a} /[n \tan (\pi / n)]}<R \cos (\pi / n)$ has to be satisfied, a circular plate with a circular rigid insert has the minimum ultimate load $P_{0}=6 M_{0}(2-\eta) R /\left[R^{3}-\left(\sqrt{S_{a} / \pi}\right)^{3}\right]$,
and the plate with an $n_{0}$-angle rigid insert has the maximum ultimate load [ $n_{0}$ is the minimum number among all values of $n$ that satisfy the inequality $\left.\sin (2 \pi / n) n / 2>S_{a} / R^{2}\right]$.

We analyze the plate motion considered with different levels of the explosive load.

1. For $0<P_{\max } \leqslant P_{0}$ (low loads), the plate remains at rest.
2. For $P_{0}<P_{\max } \leqslant P_{1}$ (medium loads), where $P_{1}$ is the load corresponding to the emergence of the zone $Z_{p}$, the plate motion follows scheme No. 1. We determine the load $P_{1}$ as follows. Differentiating Eq. (10) in time and eliminating the quantities $\ddot{\alpha}$ and $\ddot{w}_{c}$ from the resultant equality with the use of Eqs. (18) and (19), we obtain the equality

$$
\begin{equation*}
\frac{-\rho \dot{\alpha} \dot{D}}{D} \Sigma_{1}=P(t)\left[\Sigma_{2}-\frac{\rho \Sigma_{1} \Sigma_{6}}{D\left(\rho \Sigma_{4}+\rho_{a} \Sigma_{5}\right)}\right]-M_{0}(2-\eta) \Sigma_{3} \tag{28}
\end{equation*}
$$

Taking into account that $\dot{\alpha}\left(t_{0}\right)=0$ and that the equalities $P_{1}=P\left(t_{0}\right), D\left(t_{0}\right)=\max D=R-R_{1}$, and $\xi\left(t_{0}\right)=0$ are hold if the zone $Z_{p}$ appears, whereas the zones $Z_{p}$ and $Z_{3}$ are absent, we obtain

$$
\begin{equation*}
P_{1}=\frac{M_{0}(2-\eta) \Sigma_{3}(0)}{\Sigma_{2}(0)-\rho \Sigma_{1}(0) \Sigma_{6}(0) /\left\{\left(R-R_{1}\right)\left[\rho \Sigma_{4}(0)+\rho_{a} \Sigma_{5}\right]\right\}} \tag{29}
\end{equation*}
$$

It is seen from Eqs. (26) and (29) that $P_{0}<P_{1}$. We write Eq. (23) for scheme No. 1 in the form

$$
\begin{equation*}
\ddot{w}_{c}(t)=Q\left[P(t)-P_{0}\right], \tag{30}
\end{equation*}
$$

where $Q=\Sigma_{8} /\left(\rho \Sigma_{7}+\rho_{a} \Sigma_{5}\right)$. The initial conditions have the form (13). At the time $t=T$, the load is removed, and the plate moves by inertia for a certain time.

For $t_{0} \leqslant t \leqslant T$, integrating the equation of motion (30), we have

$$
\dot{w}_{c}(t)=Q\left[\int_{t_{0}}^{t} P(\tau) d \tau-P_{0}\left(t-t_{0}\right)\right], \quad w_{c}(t)=Q\left[\int_{t_{0}}^{t} \int_{t_{0}}^{m} P(\tau) d \tau d m-P_{0} \frac{\left(t-t_{0}\right)^{2}}{2}\right] .
$$

For $T<t \leqslant t_{f}$, the plate motion occurs owing to inertia until the plate stops at the time $t_{f}$; it is described by the equation

$$
\ddot{w}_{c}(t)=-Q P_{0}
$$

with the initial conditions $\dot{w}_{c}(T)$ and $w_{c}(T)$. The time $t_{f}$ is determined by the condition

$$
\begin{equation*}
\dot{w}_{c}\left(t_{f}\right)=0 \tag{31}
\end{equation*}
$$

Integrating the equation of motion, we obtain the equalities

$$
\begin{gather*}
\dot{w}_{c}(t)=\dot{w}_{c}(T)-Q P_{0}(t-T)  \tag{32}\\
w_{c}(t)=w_{c}(T)+\dot{w}_{c}(T)(t-T)-Q P_{0}(t-T)^{2} / 2
\end{gather*}
$$

It follows from Eqs. (31) and (32) that

$$
\begin{equation*}
t_{f}=t_{0}+\frac{1}{P_{0}} \int_{t_{0}}^{T} P(t) d t \tag{33}
\end{equation*}
$$

The deflections are calculated by Eqs. (4) and (22), and the maximum final flexure is found by the formula

$$
w_{c}\left(t_{f}\right)=Q\left[\frac{1}{2 P_{0}}\left(\int_{t_{0}}^{T} P(t) d t\right)^{2}-\int_{t_{0}}^{T}\left(t-t_{0}\right) P(t) d t\right]
$$

3. For high loads $P_{1}<P_{\max } \leqslant P_{2}$ ( $P_{2}$ is the load at which the zone $Z_{2}$ disappears, ) the plate motion starts with a developed zone $Z_{p}$ and $R-R_{1} / \cos (\pi / n)<D\left(t_{0}\right) \leqslant R-R_{1}$. The initial values $\xi_{0}=\xi\left(t_{0}\right)$ and $D_{0}=D\left(t_{0}\right)$ are determined from Eq. (28) with allowance for the equality $\dot{\alpha}\left(t_{0}\right)=0$ and relation (20):

$$
\begin{equation*}
P_{\max }\left\{\Sigma_{2}\left(\xi_{0}\right)-\frac{\rho \Sigma_{1}\left(\xi_{0}\right) \Sigma_{6}\left(\xi_{0}\right)}{D_{0}\left[\rho \Sigma_{4}\left(\xi_{0}\right)+\rho_{a} \Sigma_{5}\right]}\right\}=M_{0}(2-\eta) \Sigma_{3}\left(\xi_{0}\right) \tag{34}
\end{equation*}
$$



Fig. 8

The load $P_{2}$ is determined from equality (34) with $\xi_{0}=\pi / n$ and $D_{0}=R-R_{1} / \cos (\pi / n)$ :

$$
P_{2}=\frac{M_{0}(2-\eta) \Sigma_{3}(\pi / n)}{\Sigma_{2}(\pi / n)-\rho \Sigma_{1}(\pi / n) \Sigma_{6}(\pi / n) /\left\{\left[R-R_{1} / \cos (\pi / n)\right]\left[\rho \Sigma_{4}(\pi / n)+\rho_{a} \Sigma_{5}\right]\right\}} .
$$

In the first phase of deformation $\left(t_{0}<t \leqslant t_{1}\right)$, the plate motion occurs by scheme No. 2. The deformation is described by Eqs. (10) and (18)-(21) with the initial conditions (13) and (34). In this phase, the zone $Z_{p}$ is compressed $(\dot{D}>0)$ by the law described by Eq. (28). The time $t_{1}$ corresponding to disappearance of the zone $Z_{p}$ is determined from the equality $\xi\left(t_{1}\right)=0$. At this time, the values of $\dot{w}_{c}\left(t_{1}\right)$ and $w_{c}\left(t_{1}\right)$ are determined.

In the second phase of deformation $\left(t_{1}<t \leqslant t_{f}\right)$, the plate motion occurs by scheme No. 1 until the stop at the time $t_{f}$. The deformation is described by Eqs. (22) and (23) with the initial conditions determined at the end of the first phase of motion. The time of the stop is determined by condition (31). All deflections in the plate are calculated by Eqs. (4) and (20)-(22) with allowance for all phases of motion.
4. For $P_{\max }>P_{2}$ (superhigh loads), the plate motion starts by scheme No. 3 with a developed zone $Z_{p}$, which completely covers the rigid insert $Z_{a}$, and then we have $0<D<R-R_{1} / \cos (\pi / n)$. We find the value of $D_{0}=D\left(t_{0}\right)$ as follows. Differentiating Eq. (10) with respect to time and eliminating $\ddot{\alpha}$ and $\ddot{w}_{c}$ from the resultant equality with the use of Eqs. (24) and (25), we obtain

$$
\begin{equation*}
\frac{-\rho \dot{\alpha} \dot{D}}{D} \Sigma_{10}=P(t)\left[\Sigma_{11}-\frac{\rho \Sigma_{14} \Sigma_{10}}{D\left(\rho \Sigma_{13}+\rho_{a} \Sigma_{5}\right)}\right]-M_{0}(2-\eta) \Sigma_{12} \tag{35}
\end{equation*}
$$

Taking into account that $\dot{\alpha}\left(t_{0}\right)=0$, we determine $D_{0}$ from the equality

$$
\begin{equation*}
P_{\max }\left\{\Sigma_{11}\left(D_{0}\right)-\frac{\rho \Sigma_{14}\left(D_{0}\right) \Sigma_{10}\left(D_{0}\right)}{D_{0}\left[\rho \Sigma_{13}\left(D_{0}\right)+\rho_{a} \Sigma_{5}\right]}\right\}=M_{0}(2-\eta) \Sigma_{12} . \tag{36}
\end{equation*}
$$

In the first phase of deformation $\left(t_{0}<t \leqslant t_{1}\right)$, the plate motion occurs by scheme No. 3. The deformation is described by Eqs. (10), (24), and (25) with the initial conditions (13) and (36). In this phase, the zone $Z_{p}$ is compressed by the law described by Eq. (35). The time $t_{1}$ corresponding to the emergence of the zone $Z_{2}$ is determined from the equality $D\left(t_{1}\right)=R-R_{1} / \cos (\pi / n)$. At this time, the values of $\dot{\alpha}\left(t_{1}\right)$ and $\alpha\left(t_{1}\right)$ are found.

In the second $\left(t_{1}<t \leqslant t_{2}\right)$ and third $\left(t_{2}<t \leqslant t_{f}\right)$ phases of deformation, the plate motion occurs in the same manner as the first and second stages of deformation under high loads with appropriate initial values.

All deflections are calculated from Eqs. (4) with allowance for all phases of motion. The solid curves in Fig. 8 refer to the deflections $w=u R^{2} \rho /\left(M_{0} T^{2}\right)$ with $\varphi=0$ of a simply supported circular plate with a square


Fig. 9
rigid insert $(n=4)$ and $R_{1} / R=0.2$ and $\rho_{a} / \rho=3.0$ under the action of a superhigh load of a "rectangular" form: $P(t)=33.82 M_{0} / R^{2}$ (for $\left.0 \leqslant t \leqslant T\right)$ or $P(t)=0$ (for $t>T$ ). Curves $1-4$ show the deflections of the plate at the times $t=T, t=t_{1}=1.89 T, t=t_{2}=2.12 T$, and $t=t_{f}=5.88 T$, respectively.
3. As another example, we consider the dynamic behavior of an elliptical plate with a rigid insert $Z_{a}$ whose contour is located at identical distances $D_{a}$ from the external contour (Fig. 9). The equation of the contour $l$ is set in a parametric form: $x_{1}=a \cos \varphi, y_{1}=b \sin \varphi(0 \leqslant \varphi \leqslant 2 \pi$ and $b \leqslant a)$. We assume that $0<D_{a} \leqslant b^{2} / a$ and the pivot $l_{1}$ is not formed (see [6]). Then, the equation of the contour of the rigid insert $l_{2}$ has the form (1) for $D=D_{a}$. The curvilinear coordinate system (16) has the form

$$
\begin{gathered}
x=\left[a-\nu_{1} b / L\left(\nu_{2}\right)\right] \cos \nu_{2}, \quad y=\left[b-\nu_{1} a / L\left(\nu_{2}\right)\right] \sin \nu_{2}, \\
L(\varphi)=\sqrt{a^{2} \sin ^{2} \varphi+b^{2} \cos ^{2} \varphi}
\end{gathered}
$$

Two deformation schemes are possible for the plate considered. Under medium loads, the plate is deformed in a cone-shaped manner (scheme No. 1; Fig. 9). The angle of rotation of the plate around the support contour of the plate is identical for all $\varphi$ and equal to $\alpha(t)$. Under high loads, the rigid insert $Z_{a}$ is located inside the plastic region $Z_{p}$, and the contour $l_{3}$ is located at identical distances $D_{a}-D$ from the points of the contour of the rigid insert $l_{2}$ (scheme 3; Fig. 10).

The equations of motion (8) and (9) under high loads have the form of Eqs. (18) and (19), where $\Sigma_{i}$ should be replaced by $\Omega_{i}(i=1, \ldots, 6)$ :

$$
\begin{gathered}
\Omega_{1}(D)=\iint_{Z_{3}} d_{3}^{2} d s=\int_{0}^{2 \pi}\left\{\int_{0}^{D} \nu_{1}^{2}\left[L\left(\nu_{2}\right)-\frac{\nu_{1} a b}{L^{2}\left(\nu_{2}\right)}\right] d \nu_{1}\right\} d \nu_{2}=\frac{D^{3}}{6}\left[8 \int_{0}^{\pi / 2} L(\varphi) d \varphi-3 \pi D\right] \\
\Omega_{2}(D)=\iint_{Z_{3}} d_{3} d s=\int_{0}^{2 \pi}\left\{\int_{0}^{D} \nu_{1}\left[L\left(\nu_{2}\right)-\frac{\nu_{1} a b}{L^{2}\left(\nu_{2}\right)}\right] d \nu_{1}\right\} d \nu_{2}=\frac{2 D^{2}}{3}\left[3 \int_{0}^{\pi / 2} L(\varphi) d \varphi-\pi D\right] \\
\Omega_{3}=\int_{l} d l=4 \int_{0}^{\pi / 2} L(\varphi) d \varphi ; \quad \Omega_{4}(D)=\iint_{Z_{p}} d s=4\left(D_{a}-D\right) \int_{0}^{\pi / 2} L(\varphi) d \varphi-\pi\left(D_{a}^{2}-D^{2}\right), \\
\Omega_{5}=\iint_{Z_{a}} d s=\pi a b-D_{a}\left[4 \int_{0}^{\pi / 2} L(\varphi) d \varphi-\pi D_{a}\right]
\end{gathered}
$$

$$
\Omega_{6}(D)=\Omega_{4}(D)+\Omega_{5}=\pi a b-D\left[4 \int_{0}^{\pi / 2} L(\varphi) d \varphi-\pi D\right]
$$

Here $d_{3}(x, y)$ is the distance from the point $(x, y)$ to the support contour; $Z_{3}=S \backslash\left(Z_{a} \cup Z_{p}\right)$. Then, the plate motion is described by Eqs. (10), (18), and (19) with allowance for the replacement made.

The equations of motion (14) and (15) under medium loads have the form

$$
\dot{\alpha} D_{a}=\dot{w}_{c} ; \quad \ddot{w}_{c}\left[\rho \frac{\Omega_{1}\left(D_{a}\right)}{D_{a}^{2}}+\rho_{a} \Omega_{5}\right]=P(t)\left[\frac{\Omega_{2}\left(D_{a}\right)}{D_{a}}+\Omega_{5}\right]-M_{0}(2-\eta) \frac{\Omega_{3}}{D_{a}}
$$

The ultimate load (17) with allowance for $\int_{0}^{\pi / 2} L(\varphi) d \varphi \approx \frac{\pi}{8}[3(a+b)-2 \sqrt{a b}]$ (see [6]) is calculated by the formula

$$
P_{0}=\frac{M_{0}(2-\eta) \Omega_{3}}{\Omega_{2}\left(D_{a}\right)+D_{a} \Omega_{5}} \approx \frac{6 M_{0}(2-\eta)[3(a+b)-2 \sqrt{a b}]}{D_{a}\left\{4\left(3 a b+D_{a}^{2}\right)-3 D_{a}[3(a+b)-2 \sqrt{a b}]\right\}}
$$

in the case of a circular plate with a circular rigid insert ( $a=b=R$ and $D_{a}=R-R_{1}$ ), it coincides with Eq. (27).
An analysis of the dynamic behavior of the plate considered is similar to the analysis performed above for a circular plate with a regular polygonal rigid insert. The difference is that $P_{1}=P_{2}$ and scheme No. 1 is realized after scheme No. 3 for $P_{\max }>P_{1}$. The load $P_{1}$ is calculated by the formula [see Eq. (29)]

$$
P_{1}=\frac{M_{0}(2-\eta) \Omega_{3}}{\Omega_{2}\left(D_{a}\right)-\frac{\rho \Omega_{1}\left(D_{a}\right) \Omega_{6}\left(D_{a}\right)}{D_{a}\left[\rho \Omega_{4}\left(D_{a}\right)+\rho_{a} \Omega_{5}\right]}} \approx \frac{6 M_{0}(2-\eta) \rho_{a}[3(a+b)-2 \sqrt{a b}]}{\rho D_{a}^{2}\left[3(a+b)-2 \sqrt{a b}-2 D_{a}\left(4 \rho_{a} / \rho-3\right)\right]}
$$

The load $p_{0}=P_{0} a^{2} /\left[(2-\eta) M_{0}\right]$ as a function of the ratio $D_{a} / a$ for different values of $b / a$ is plotted in Fig. 7: curve 4 refers to $b=a$, curve 5 refers to $b / a=0.8$, and curve 6 refers to $b / a=0.6$. The dashed and dotted curves in Fig. 8 correspond to the deflections $w=u a^{2} \rho /\left(M_{0} T^{2}\right)$ in the cross section $y=0$ of a simply supported elliptic plate with $b / a=0.8$, with a rigid insert and $D_{a} / a=0.5, \rho_{a} / \rho=1.5$ under the action of a high load with linear attenuation: $P(t)=75.48(T-t) M_{0} / R^{2}$ for $0 \leqslant t \leqslant T$ and $P(t)=0$ for $t>T$. Curves 5-7 show the deflections of the plate at the times $t=T, t=t_{1}=1.16 T$, and $t=t_{f}=4.6 T$, respectively.

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